

## Request for approval of engineering plans and reports required by conditions of Resource Consent or Consent Notice

### 1. Applicant details (Consent Holder or Property Owner):

Name/s: (please write all  
names in full)

Dandy Developments Ltd

Phone numbers:

Work:

[REDACTED]

Home:

Email:

[REDACTED]



**Check this box if the you wish to be included in correspondence regarding this application.**

### 2. Designer/Engineer contact details (Contact Person):

Name/s: (please write all  
names in full)

[REDACTED]

Contact phone number:

[REDACTED]

Email:

[REDACTED]

### 3. Primary Contact details:

(Please specify which person identified above is the primary point of contact):

Name/s:

Sebastian Hicks

### 4. The document number for which these plan(s) or report(s) relate:

Please note the Resource Consent or Consent Notice reference below. Please ensure the correct suffix is used if there are any variations or objections associated with this application.

2230063-RMASUB

### 5. Conditions to be approved:

List the conditions to which this request relates and specify which documents relate to each.

If additional space is required, please attach a summary document.

Condition	Document reference	Drawing numbers
3(d)(i)	Geologix	300 & 390
3(d)(ii)	ENGINEERING REPORT	Section 5, Sheets 300, 250 & 380
3(d)(iii)	FOR	Section 4, Sheets 110, 380 & 390
3(d)(iv)	ENGINEERING	
	DESIGN APPROVAL	Section 6, 6.1, Sheet 130

6. Does this application include any of the following (tick where appropriate):

Infrastructure:	To be vested:	Upgrades:	New connection:
Roads			
Street lighting			
Wastewater	Note - Lot 7 to vest in FNDC for sewerage		but no wastewater assets
Stormwater			
Potable Water			

7. Please note any associated Building Consent reference (if applicable):

N/A

9. Billing details:

This identifies the person or entity that will be responsible for paying any invoices or receiving any refunds associated with processing this request for approval of engineering plans and reports. Staff time required to process this approval will be charged on completion of the work. Please also refer to the council's Fees and Charges document (available at [www.fndc.govt.nz](http://www.fndc.govt.nz)). A deposit is payable when you submit this request.

Name/s: (please write all names in full)

Dandy Developments

Postal address:



Post code:



Phone numbers:

Work:



Home:

Fax:

Email:



Name of bill payer: Danielle Moa (please print)

Signature: (signature of bill payer – mandatory)

Date:



Important information:

**Privacy information:** Once this application is lodged with the council it becomes public information. Please advise us if there is sensitive information included in this request. The information you have provided on this form is required so that your application for approval can be processed. The information will be stored on the council's property files and held by the Far North District Council and will be made available on request.

**Declaration:** The information I have supplied with this application is true and complete to the best of my knowledge.

Name: Danielle Moa (please print)

Signature: (signature)

Date:





**geologix**  
consulting engineers

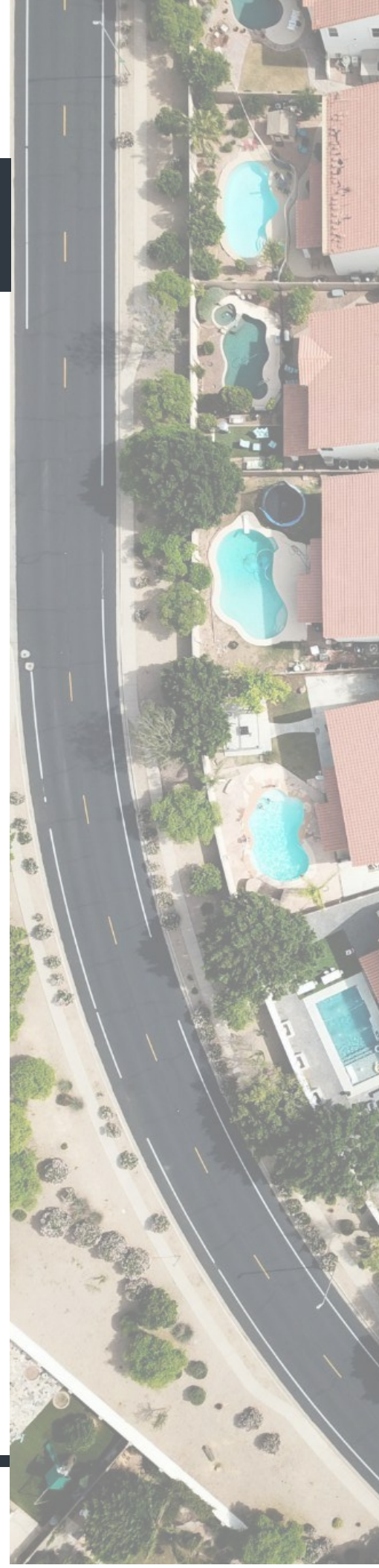
# ENGINEERING REPORT

## FOR ENGINEERING DESIGN APPROVAL (EDA)

458A HIHI ROAD, HIHI

MOA EQUITY LTD


**C0651N-S02  
AUGUST 2025  
REVISION 1**





**geologix**  
consulting engineers

## DOCUMENT MANAGEMENT

<b>Document Title</b>	Engineering Report – For Engineering Design Approval (EDA)
<b>Site Reference</b>	458A Hihi Road, Hihi
<b>Client</b>	Moa Equity Ltd
<b>Geologix Reference</b>	C0651N-S02
<b>Issue Date</b>	August 2025
<b>Revision</b>	01
<b>Prepared by</b>	Gerard McHardy Civil Engineer, BE Civil 
<b>Reviewed</b>	Sebastian Hicks Principal Civil Engineer, BSc Eng (Hons), CPEng Reg. 1168062, IntPE(NZ) /APEC Engineer, CMEngNZ
<b>Approved by</b>	Edward Collings Managing Director, CEnvP Reg. 0861, CPEng Reg. 1033153, CMEngNZ

## REVISION HISTORY

Date	Issue	Prepared	Reviewed	Approved
August 2025	For EDA Application	GM	SH	EC





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## 1 INTRODUCTION

This Engineering Report has been prepared by Geologix Consulting Engineers Ltd (Geologix) for Moa Equity Ltd as our Client in accordance with our standard short form agreement and general terms and conditions of engagement.

The scope of works has been undertaken to prepare an Engineering Design Approval (EDA) application in relation to the proposed subdivision of 458A Hihi Road, Hihi (the 'site') into 7 new Lots. Refer Figure 1 and 2 for images of the proposed site layout.

The proposed subdivision has been granted Resource Consent by Far North District Council (FNDC). The FNDC Resource Consent reference is 2230063-RMASUB. The consents are based on supporting information provided by others. This documentation is noted below for reference and included in the appendices of this report where indicated:

- Scheme Plan, Ref. No 23812, Proposed Subdivision of Lot 2 DP 195378, Dated May 2022, by Williams and King Ltd, stamped "Approved Plan" by FNDC 22/04/2024 – Refer Appendix A
- Site Suitability Report for Proposed Subdivision, Ref 22118, Revision Final, Dated May 2022, by Haigh Workman Ltd Civil and Structural Engineers<sup>1</sup>

The detailed design presented with this EDA application intends to only advance the necessary elements of the preliminary design to satisfy the related conditions of consent. This report serves to provide clarity to the adopted design and drawings included in Appendix B.

The focus of the detailed design is the accessway (Right of Way) and stormwater drainage to facilitate the subdivision to the satisfaction of the related conditions of consent. All other on-lot infrastructure (such as wastewater and water supply) is to be applied for under the individual on-lot development consents in accordance with the conditions of the above-referenced resource consent.

It is also noted that the vehicle crossings to each lot from the Right of Way are indicative within in this submission. These crossings are proposed to be finalised and formed at time of individual lot development, rather than at subdivision formation.

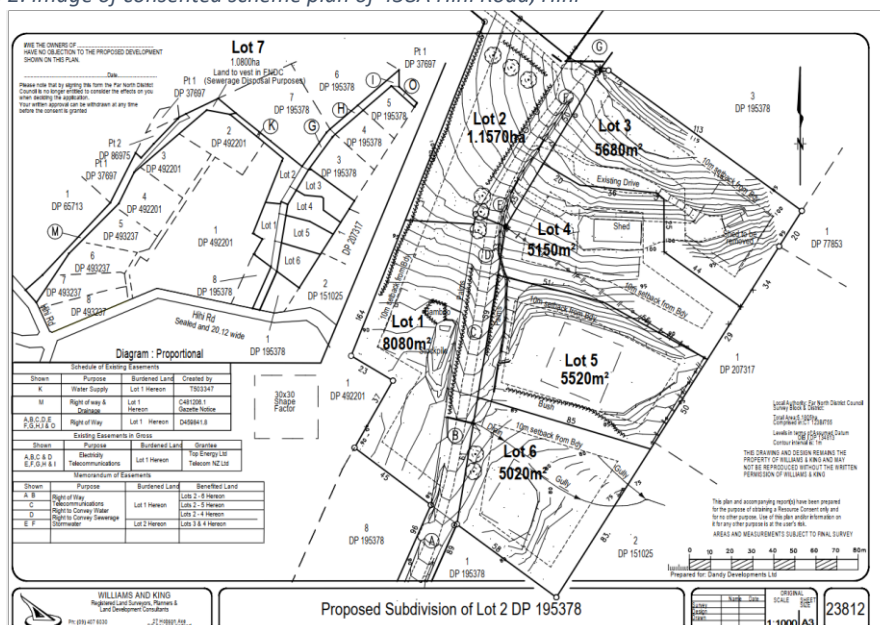
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<sup>1</sup> Site Suitability Report for Proposed Subdivision, Ref 22118, Revision Final, Dated May 2022, by Haigh Workman Ltd Civil and Structural Engineers

Figure 1: Aerial Image of 458A Hihi Road, Hihi



Figure 2: Image of consented scheme plan of 458A Hihi Road, Hihi



## 2 SITE DESCRIPTION

The site is located on Hihi Road, Hihi, on the northern side of the road. The site encompasses 51,772 m<sup>2</sup>, including access road corridors, with the gross area of approximately 35,000m<sup>2</sup> available for the subdivision. The majority of the site is grass with some areas of bush along the eastern side. There is an existing unconsented shed in the north-eastern quadrant of the

site which will be removed as part of the development. The topography of the site is generally gently rising to the north from Hihi Road with an unsealed access road following a ridge along the western side of the site. In the south-eastern corner of the site there is a gully trending southeast towards an estuary feeding the Mangonui Harbour.

As indicated on FNDC Maps and in Figure 3 below, there is no existing 3 water infrastructure or reticulated networks present within the site boundaries.

*Figure 3: FNDC 3Waters Maps GIS Image of Existing Services*



### 3 PROPOSED DEVELOPMENT

The approved scheme plan<sup>2</sup> received from Williams and King presents a subdivision comprising 6 residential Lots and an access road (Lot 7) to be vested in Council.

It is proposed to upgrade the existing vehicle crossing off Hihi Road which complies with the Councils Engineering Standard FNDC/S/6D and section 3.3.7.1 of the Engineering standards and NZS4404:2004 in accordance with the plans approved under Condition 3(b) of the Resource Consent. Vegetation is to be cleared in both directions along Hihi Road in accordance with the recommendations in Haigh Workman report<sup>3</sup>, reference 22 118, Chapter 5.6.

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<sup>2</sup> Scheme Plan, Ref. No 23812, Proposed Subdivision of Lot 2 DP 195378, Dated May 2022, by Williams and King Ltd

<sup>3</sup> Site Suitability Report for Proposed Subdivision, Ref 22118, Revision Final, Dated May 2022, by Haigh Workman Ltd Civil and Structural Engineers



The existing access road will be upgraded to a sealed access ROW with a 5m wide carriageway in accordance with condition 4(b) of the Resource Consent, noting this is a departure from Table 3-16 of the FNDC Engineering Standards.

The formation is to consist of a minimum of 150mm of GAP 65 sub-base and 100mm GAP 40 basecourse with a sealed surface and is to include water table drains, culverts and culvert wingwalls as required to direct and control stormwater runoff.

Stormwater considerations are detailed further in Section 4.

The site has minimal earthworks proposed and is only required to implement the above roading and drainage.

## 4 STORMWATER

### 4.1 Existing (Pre-development) Considerations

Stormwater runoff on the eastern side of the existing accessway generally flows towards a gully within the proposed Lot 6, trending south-eastwards through a neighbouring property to the CMA.

Runoff from the accessway and proposed Lots 1 and 2 is generally directed to swale drains alongside the accessway and discharged towards the gully, with a 300mm culvert under the accessway directing runoff from the western side towards the gully.

To the south of this culvert, stormwater is directed to swale drains on either side of the accessway carry runoff to the Hihi Road water table. Two culverts carry the runoff across the road towards the estuary via a neighbouring property.

At the northern end of the accessway, there is a 250mm culvert that crosses the ROW and redirects the runoff to the west.

It is noted that sediment and vegetation has caused many of the intakes to be partially blocked.

### 4.2 Proposed Stormwater Management

The stormwater management elements directly addressed within this Engineering Approval application are:

- Catchment post-development flows for the proposed subdivision, determined by the rational method.
- Sizing of roadside swales.
- Routing of drainage, considering the new paved ROW and the effect on the existing drainage paths.
- Sizing of culverts under the right of way and proposed vehicle crossings.





Key design criteria for stormwater management elements are presented as follows:

- The stormwater discharge from the site is determined for a design storm of 10% AEP.
- Rainfall data has been obtained from NIWA HIRDS V4.0, for the 10yr ARI / 10% AEP, 10-minute storm duration, adjusted for a 20% increase due to climate change as per FNDC engineering standards.

The site has been divided into five catchments.

Catchment A encompasses approximately 5,380 m<sup>2</sup>, incorporating most of Lots 1 and 2 and the western side of the proposed RoW. Stormwater is directed towards the western swale drain the passes through a culvert towards the gully within Lot 6, ultimately being directed to the CMA.

Catchment B encompasses approximately 1,705 m<sup>2</sup>, incorporating the eastern side of the proposed RoW and a thin strip of adjacent land. Stormwater is directed towards the eastern swale drain then through the gully within Lot 6, ultimately being directed to the CMA.

Catchment C encompasses approximately 30,142 m<sup>2</sup>, incorporating land on the eastern side of the proposed RoW. Stormwater is directed towards the gully within Lot 6, ultimately being directed to the CMA.

Catchment D encompasses approximately 5,724 m<sup>2</sup>, incorporating the balance of Lot 1 and a portion of the neighbouring site and the western side of the proposed RoW. Stormwater is directed towards the Hihi Road water table then through a culvert under Hihi Road towards the CMA via a neighbouring property.

Catchment E encompasses approximately 942 m<sup>2</sup>, incorporating the eastern side of the proposed RoW and a thin strip of adjacent land. Stormwater is directed towards the Hihi Road water table then through a culvert under Hihi Road towards the CMA via a neighbouring property.

The overall primary stormwater discharge for the proposed RoW will be managed through roadside water tables. These will be piped below road crossings and driveways where required.

The slope of the RoW generally exceeds 10%, as such all swales are to be rock lined.

All inlets are to have headwalls with screens to prevent blockage from vegetation.

There is no need to build wingwalls on the downstream side of the culverts as the swale is to be rock lined.

On lot development stormwater management will be provided as per the conditions of consent.

Engineering Calculations can be found in Appendix C.

#### 4.3 Stormwater Attenuation

As there are no downstream properties below the site, and the receiving infrastructure/channels have sufficient capacity to convey the design storms to the coastal marine area, attenuation of accessway runoff for the formed subdivision is not required.

## 5 ROADING

### 5.1 Internal Roding

The internal roading layout and details have been prepared in accordance with the conditions of consent as supported by the RC Engineering Report.

For purposes of clarification:

The roading cross-section has been prepared in accordance with the FNDC engineering standards Sheet 9, Rural Privateway Details.

The existing driveway will be upgraded to a sealed access ROW with a 5m wide carriageway along its' full length in accordance with condition 4(b) of the Resource Consent, noting this is a departure from Table 3-16 of the FNDC Engineering Standards.

The RoW will have dual crossfall throughout the full length, with a standard crossfall of 3%. The formation is to consist of a minimum of 150mm of GAP 65 sub-base and 100mm GAP 40 basecourse with a sealed surface and is to include water table drains, culverts and culvert wingwalls as required to direct and control stormwater runoff.

Longitudinal grades exceed 5%, as such the table drains will be rock lined. This will occur along the full length of the RoW.

## 6 EARTHWORKS

Earthworks are required to form the proposed internal roading, drainage channels and the stormwater pond.

The proposed earthworks activities have been modelled with 3d design by Geologix. Proposed earthworks are summarised below within Table 2. The cut volumes presented in this volume assessment includes topsoil stripping volume and fill volumes include for the pavement layer works. Earthworks for vehicle crossings has not been modelled and is anticipated to involve topsoil strip only.

*Table 2: Summary of Proposed Earthworks*

Activity	Proposed Volume	Net Volume	Total Area	Max. Height
Cut	330m <sup>3</sup>	40m <sup>3</sup>		1.0m
Fill	290m <sup>3</sup>			0.8m
<b>Total</b>	<b>m<sup>3</sup></b>	<b>40m<sup>3</sup>cut</b>	<b>2,670m<sup>2</sup></b>	

## 6.1 Erosion and Sediment Control

Erosion and sediment control measures are required to control sediment runoff from areas of proposed earthworks. These include:

- Stabilised entrance formed at the proposed crossings to Hihi Road and Brunton Place.
- Silt fences along the downslope face of the proposed RoW.
- Temporary diversion of existing overland flow paths, i.e. drainage ditches around culvert

## 7 LIMITATIONS

This report has been prepared for Moa Equity Limited as our Client. It may be relied upon by our Client and their appointed Consultants, Contractors and for the purpose of Consent as outlined by the specific objectives in this report. This report and associated recommendations, conclusions or intellectual property is not to be relied upon by any other party for any purpose unless agreed in writing by Geologix Consulting Engineers Ltd and our Client. In any case the reliance by any other party for any other purpose shall be at such parties' sole risk and no reliability is provided by Geologix Consulting Engineers Ltd.

The opinions and recommendations of this report are based on plans, specifications and reports provided to us at the time of writing, as referenced. Any changes, additions or amendments to the project scope and referenced documents may require an amendment to this report and Geologix Consulting Engineers should be consulted. Geologix Consulting Engineers Ltd reserve the right to review this report and accompanying plans.

## APPENDIX A

### Scheme Plan

I/WE THE OWNERS OF .....  
HAVE NO OBJECTION TO THE PROPOSED DEVELOPMENT  
SHOWN ON THIS PLAN.

.....Date.....  
Please note that by signing this form the Far North District  
Council is no longer entitled to consider the effects on you  
when deciding the application.  
Your written approval can be withdrawn at any time  
before the consent is granted

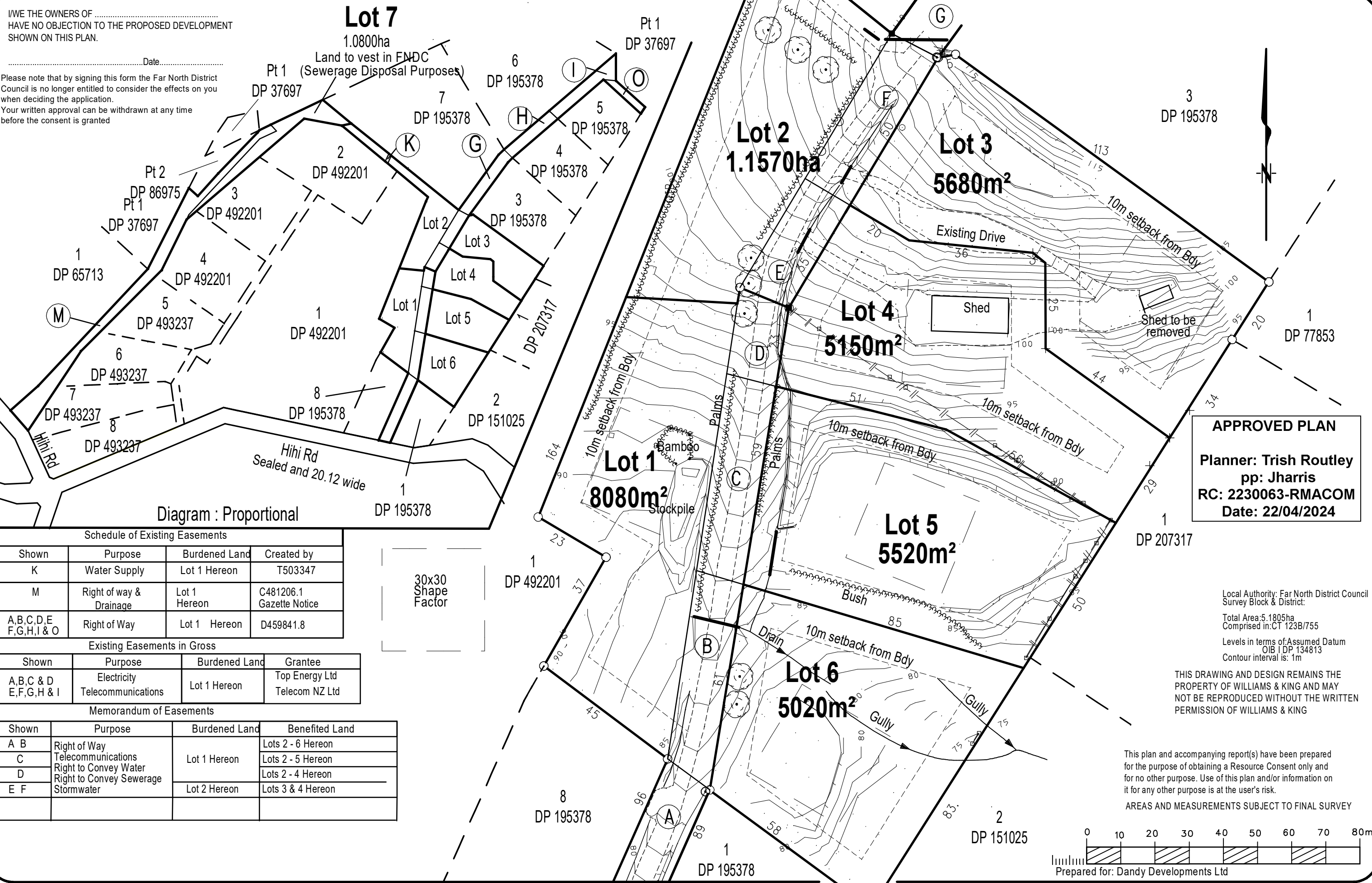


Diagram : Proportional

Schedule of Existing Easements

Shown	Purpose	Burdened Land	Created by
K	Water Supply	Lot 1 Hereon	T503347
M	Right of way & Drainage	Lot 1 Hereon	C481206.1 Gazette Notice
A,B,C,D,E F,G,H,I & O	Right of Way	Lot 1 Hereon	D459841.8

Existing Easements in Gross

Shown	Purpose	Burdened Land	Grantee
A,B,C & D E,F,G,H & I	Electricity Telecommunications	Lot 1 Hereon	Top Energy Ltd Telecom NZ Ltd

Memorandum of Easements

Shown	Purpose	Burdened Land	Benefited Land
A B	Right of Way	Lot 1 Hereon	Lots 2 - 6 Hereon
C	Telecommunications		Lots 2 - 5 Hereon
D	Right to Convey Water	Lot 2 Hereon	Lots 2 - 4 Hereon
E F	Right to Convey Sewerage Stormwater		Lots 3 & 4 Hereon

APPROVED PLAN

Planner: Trish Routley  
pp: Jharris  
RC: 2230063-RMACOM  
Date: 22/04/2024

Local Authority: Far North District Council  
Survey Block & District:

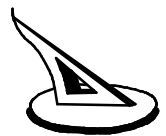
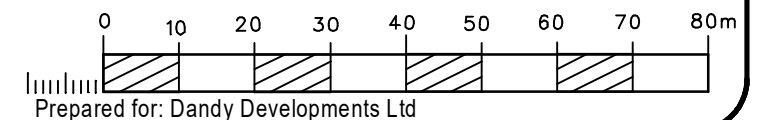
Total Area: 5.1805ha  
Comprised in: CT 123B/755

Levels in terms of: Assumed Datum  
OIB 1 DP 134813  
Contour interval is: 1m

THIS DRAWING AND DESIGN REMAINS THE  
PROPERTY OF WILLIAMS & KING AND MAY  
NOT BE REPRODUCED WITHOUT THE WRITTEN  
PERMISSION OF WILLIAMS & KING

This plan and accompanying report(s) have been prepared  
for the purpose of obtaining a Resource Consent only and  
for no other purpose. Use of this plan and/or information on  
it for any other purpose is at the user's risk.

AREAS AND MEASUREMENTS SUBJECT TO FINAL SURVEY



**WILLIAMS AND KING**  
Registered Land Surveyors, Planners &  
Land Development Consultants

Ph: (09) 407 6030  
Email: kerikeri@saps.co.nz

27 Hobson Ave  
PO Box 937 Kerikeri

Proposed Subdivision of Lot 2 DP 195378

Survey	Name	Date	ORIGINAL SCALE	SHEET SIZE
Design				
Drawn				
Rev	1	May 2022	1:1000	A3

23812



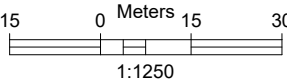
## APPENDIX B

### Drawings



GENERAL NOTES

1. DRAWING REPRODUCED FROM WILLIAMS AND KING PROPOSED SCHEME PLAN REF. 23812, DATED MAY 2022
2. CONTOURS AT 1.0 m INTERVALS.
3. TOPOGRAPHIC SURVEY DATA PROVIDED BY GEOLOGIX DRONE SURVEY
- VERTICAL DATUM NZVD2016
- CRS:NZTM2000.



2	DETAIL DESIGN	07/2025
1	CONSENT	11/05/2022
Revision	Issue	Date



AUCKLAND | NORTHLAND

Project Name and Address  
458A HIHI ROAD  
HIHI, FAR NORTH  
SUBDIVISION OF LOT 1 DP492201

Project C0458A	Drawn By TV
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Client  
MOA EQUITY LTD

Sheet Title  
PROPOSED SCHEME PLAN

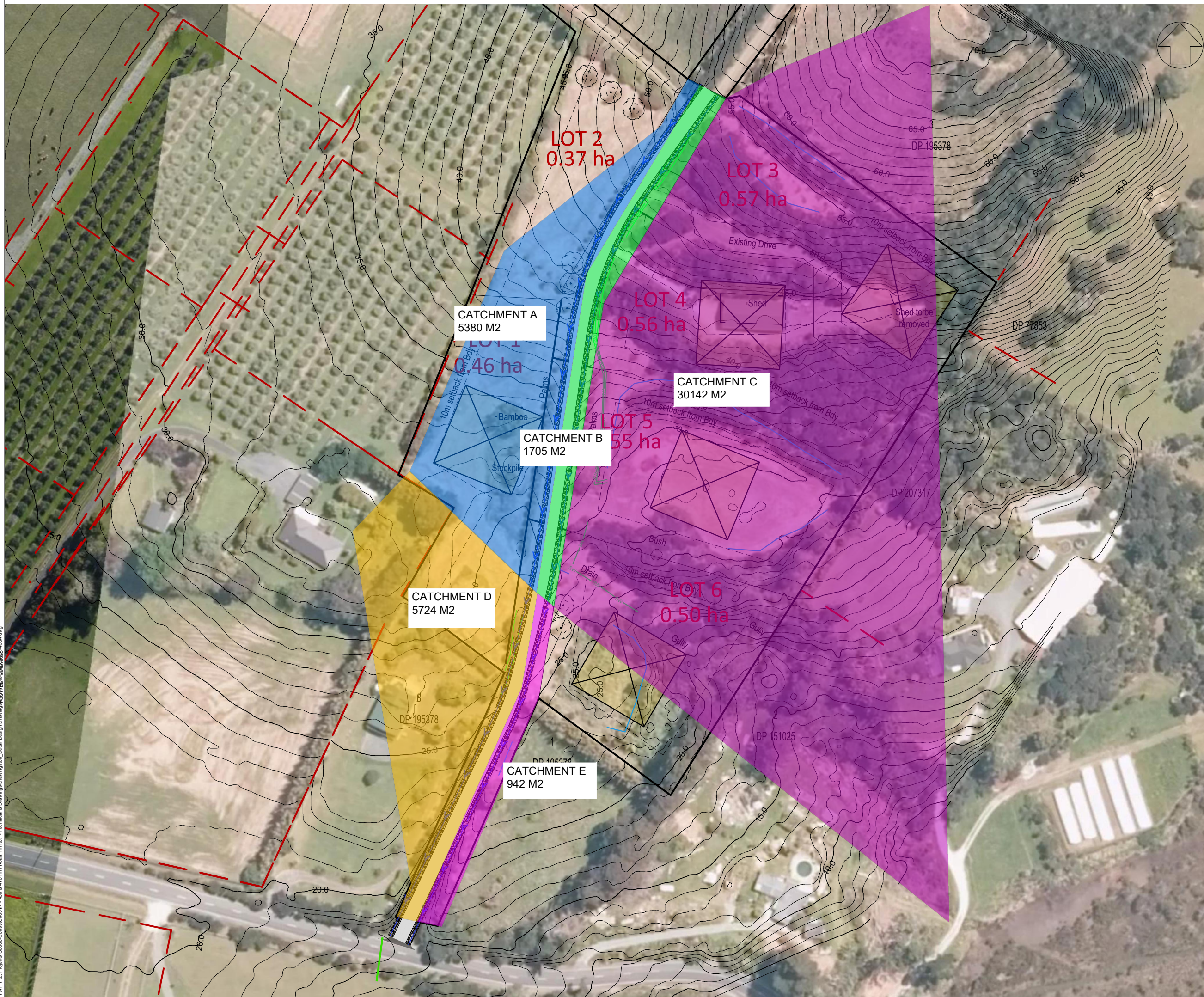
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## GENERAL NOTES

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3. TOPOGRAPHIC SURVEY DATA PROVIDED BY GEOLOGIX DRONE SURVEY  
-VERTICAL DATUM NZVD2016  
-CRS:NZTM2000.


CATCHMENT A

CATCHMENT B

CATCHMENT C

CATCHMENT D

CATCHMENT E

 WATERCOURSE  
 OVERLAND FLOWPATH  
 EXISTING DRAINAGE



2	DETAIL DESIGN	07/2025
1	CONSENT	11/05/2022
Revision	Issue	Date



AUCKLAND | NORTHLAND

Project Name and Address

458A HIHI ROAD

HIHI, FAR NORTH

SUBDIVISION OF LOT 1 DP492201

Project C0458A	Drawn By TV
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Client	
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MOA EQUITY LTD

Sheet Title

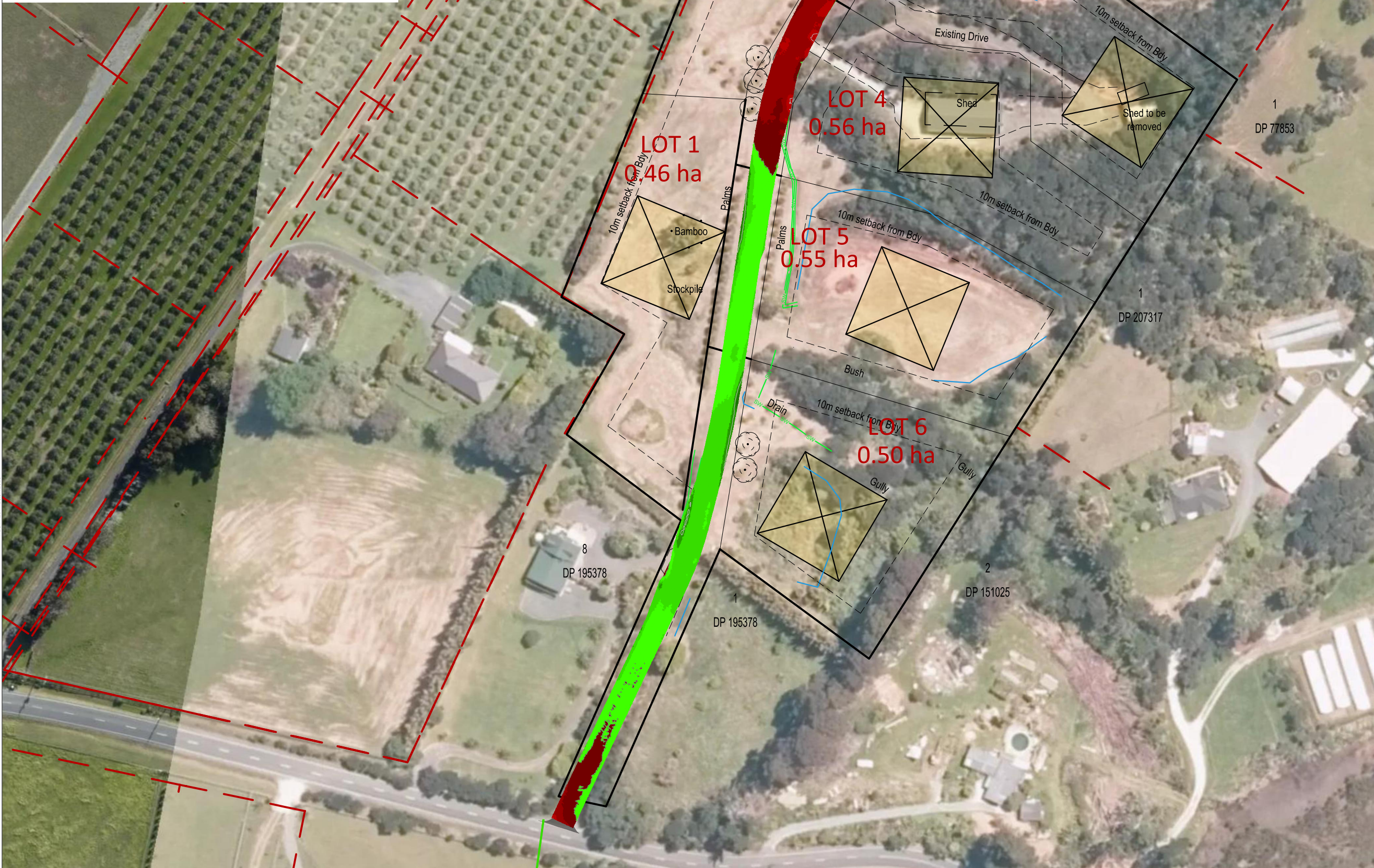
## CATCHMENT PLAN

Sheet

110



CUT AND FILL SUBGRADE COMPARED TO EXISTING GROUND			
Number	Color	Minimum Elevation (m)	Maximum Elevation (m)
1	Red	-1.00	-0.80
2	Red	-0.80	-0.60
3	Red	-0.60	-0.40
4	Dark Red	-0.40	-0.20
5	Dark Red	-0.20	0.00
6	Bright Green	0.00	0.20
7	Bright Green	0.20	0.40
8	Green	0.40	0.60
9	Green	0.60	0.80



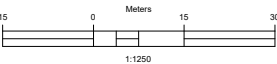
GENERAL NOTES

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- 2. CONTOURS AT 1.0 m INTERVALS.
- 3. TOPOGRAPHIC SURVEY DATA PROVIDED BY GEOLOGIX DRONE SURVEY  
-VERTICAL DATUM NZVD2016  
-CRS:NZTM2000.

EARTHWORK QUANTITIES

SUBGRADE SURFACE COMPARED TO EXISTING GROUND		
STAGE & ACTIVITY	VOLUME	NET
STAGE 2 CUT	330m³	40m³ FILL
STAGE 2 FILL	290m³	
TOTAL EARTHWORKS AREA - 2670 m²		

CUT INCLUDES TOPSOIL STRIP VOLUME  
FILL INCLUDES PAVEMENT LAYER WORKS



2	DETAIL DESIGN	07/2025
1	CONSENT	11/05/2022
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AUCKLAND | NORTHLAND

Project Name and Address  
**458A HIHI ROAD**  
**HIHI, FAR NORTH**  
SUBDIVISION OF LOT 1 DP492201

Project C0458A	Drawn By TV
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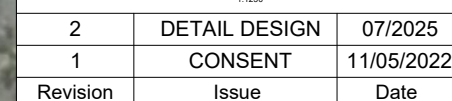
Client  
MOA EQUITY LTD

Sheet Title  
EARTHWORKS PLAN

Sheet  
**130**



- CULVERT PIPE
- OVERLAND FLOW PATH



458A HIHI ROAD  
HIHI, FAR NORTH  
SUBDIVISION OF LOT 1 DP492201

Drawn By	TV
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Sheet Title

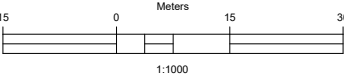
PROPOSED ROW PLAN

300

PROPOSED VEHICLE  
CROSSING AS PER  
FNDC /S/6 D  
REFER SHEET 390



GENERAL NOTES



2	DETAIL DESIGN	07/2025
1	CONSENT	11/05/2022
Revision	Issue	Date



AUCKLAND | NORTHLAND

Project Name and Address

458A HIHI ROAD

HIHI, FAR NORTH

SUBDIVISION OF LOT 1 DP492201

Project	Drawn By
C0458A	TV

Client

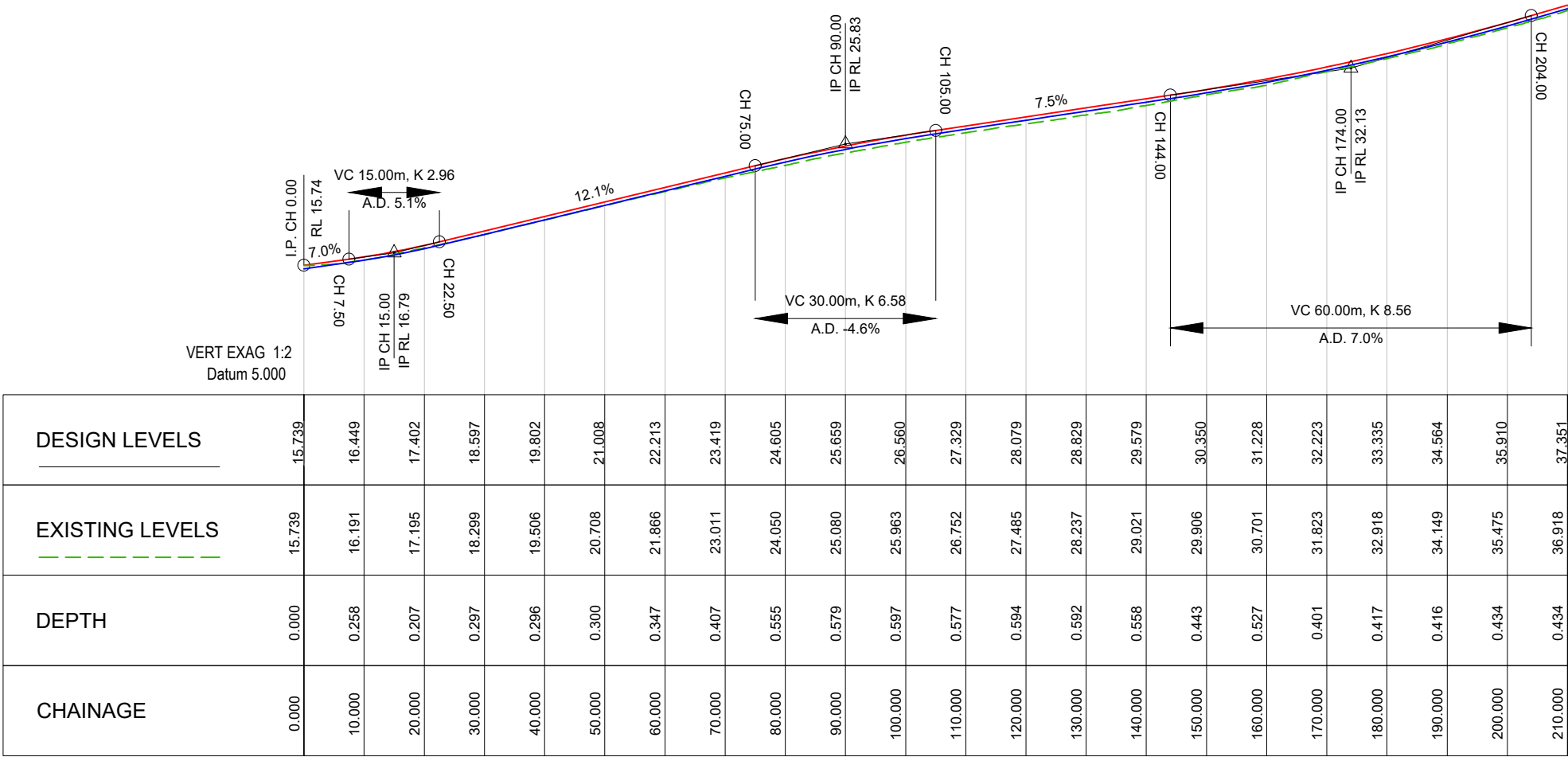
MOA EQUITY LTD

Sheet Title

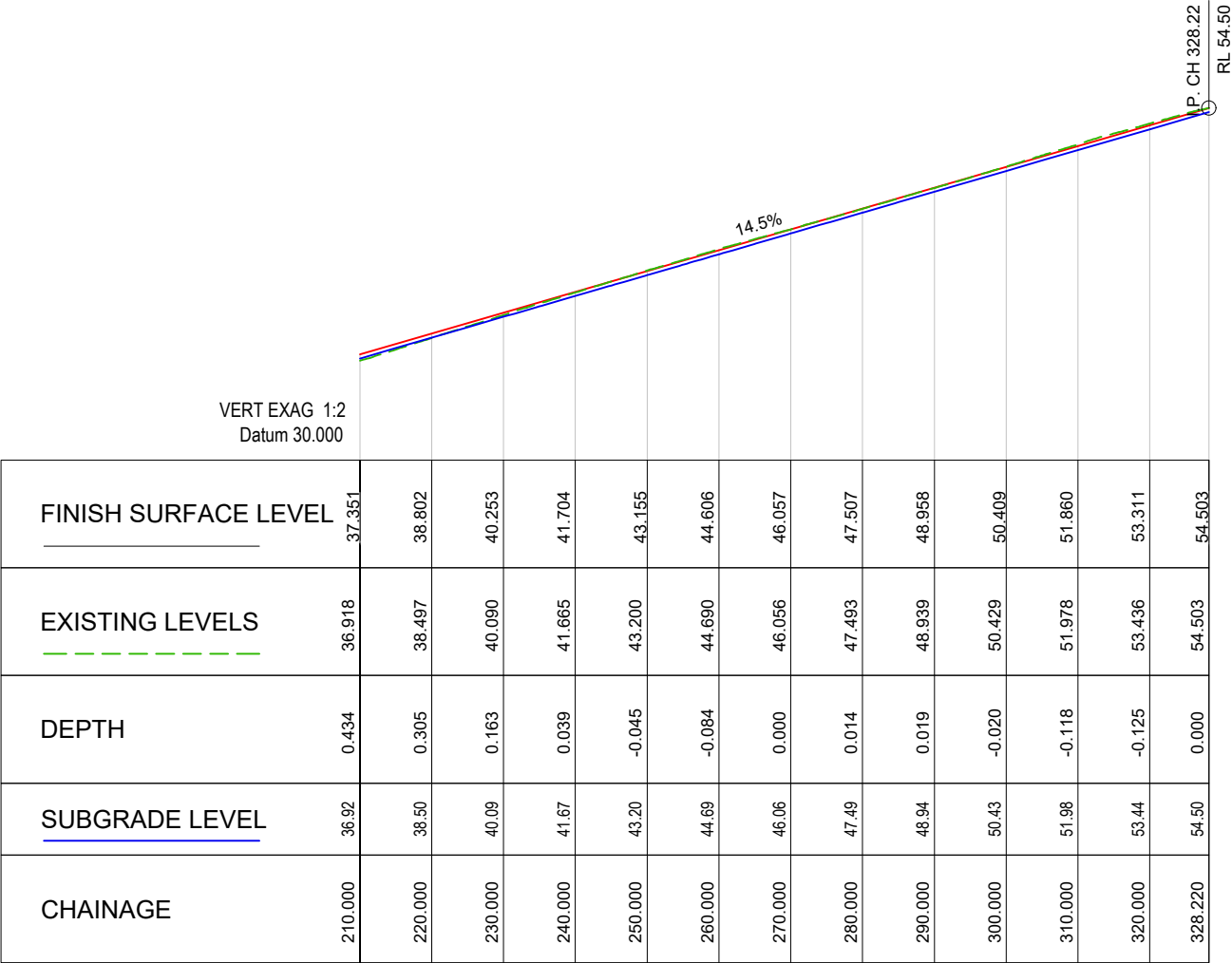
ROW LONG SECTION

Sheet

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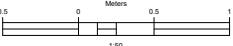
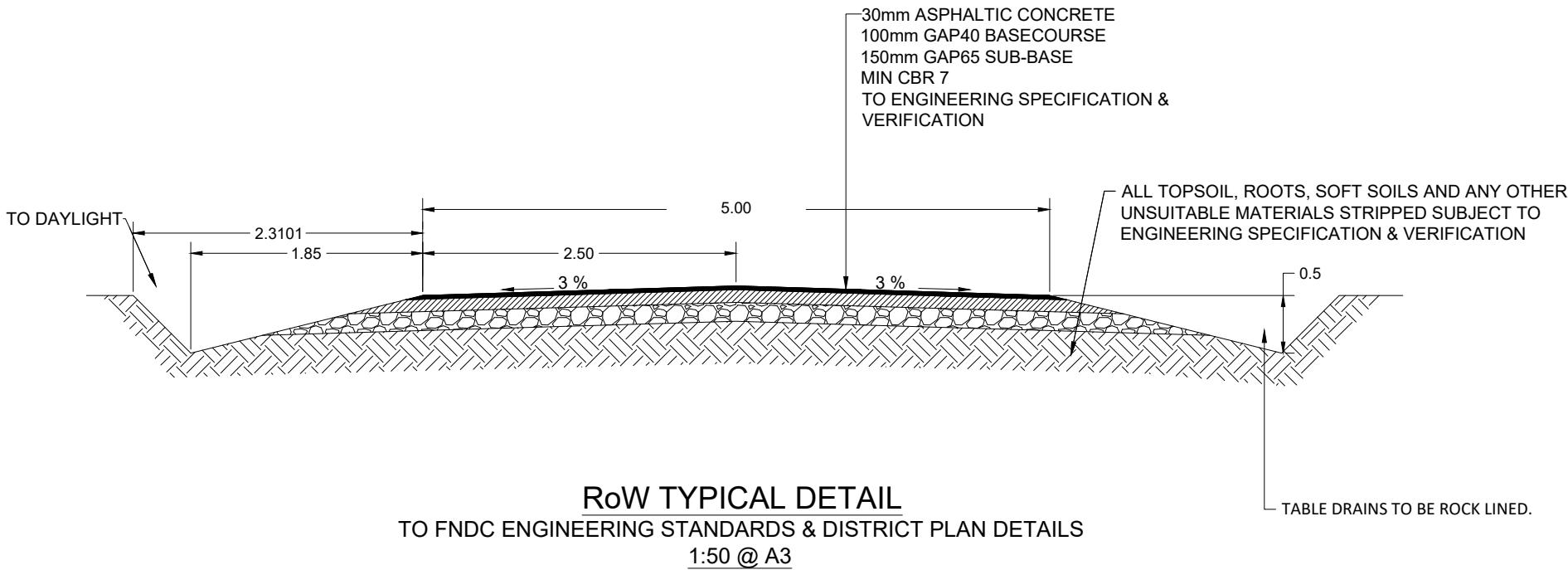


ROW LONG SECTION



ROW LONG SECTION

GENERAL NOTES



2	DETAIL DESIGN	07/2025
1	CONSENT	11/05/2022
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AUCKLAND | NORTHLAND

Project Name and Address  
458A HIHI ROAD  
HIHI, FAR NORTH  
SUBDIVISION OF LOT 1 DP492201

Project C0458A	Drawn By TV
-------------------	----------------

Client  
MOA EQUITY LTD

Sheet Title  
TYPICAL CROSS SECTIONS

Sheet  
380

GENERAL NOTES

Project Name and Address  
458A HIHI ROAD  
HIHI, FAR NORTH  
SUBDIVISION OF LOT 1 DP492201

Project  
C0651

Drawn By  
TV

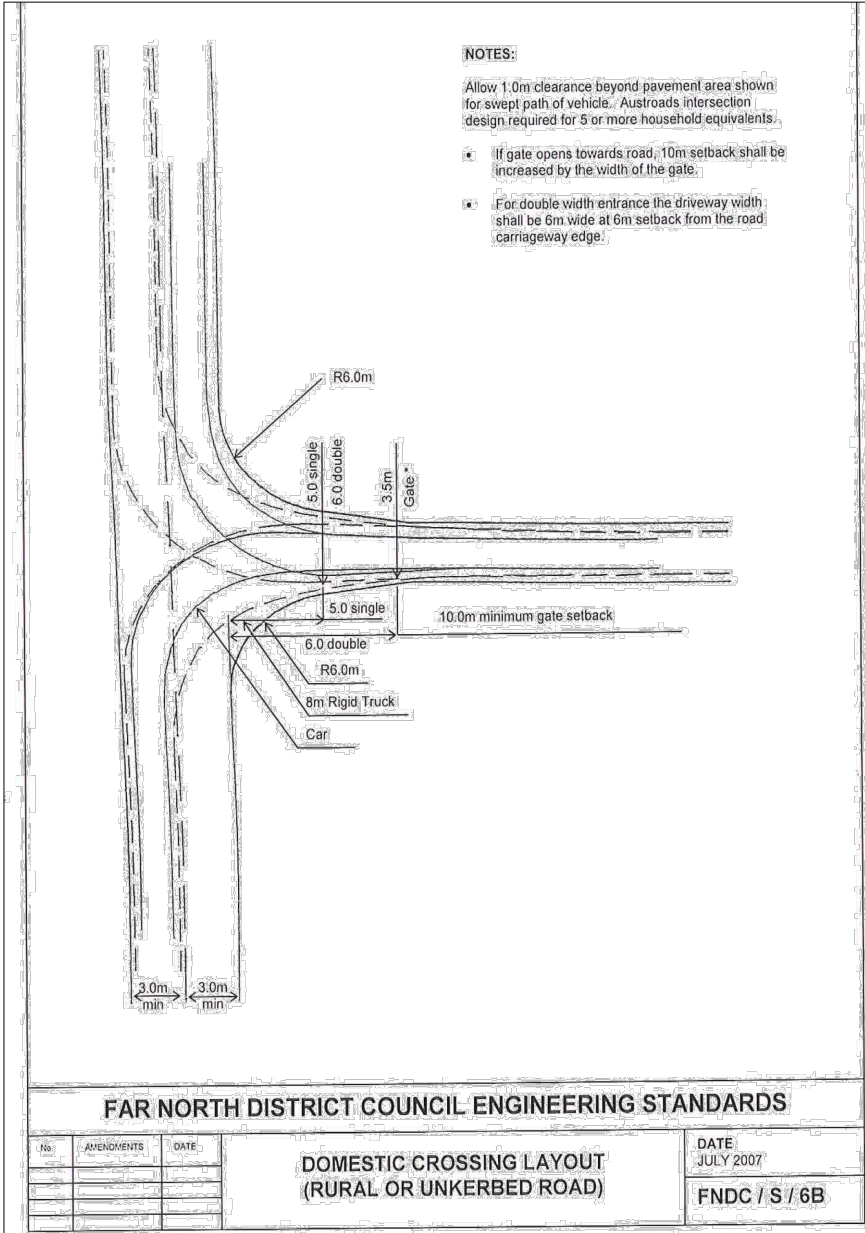
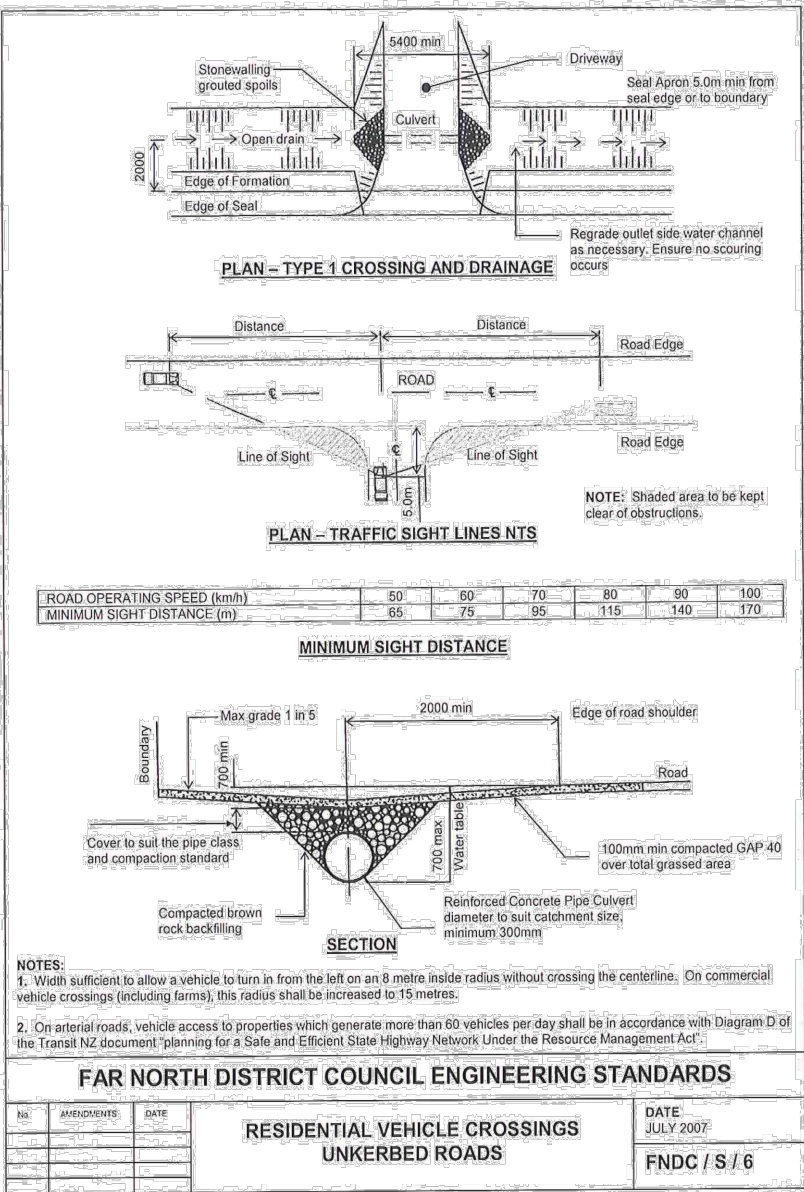
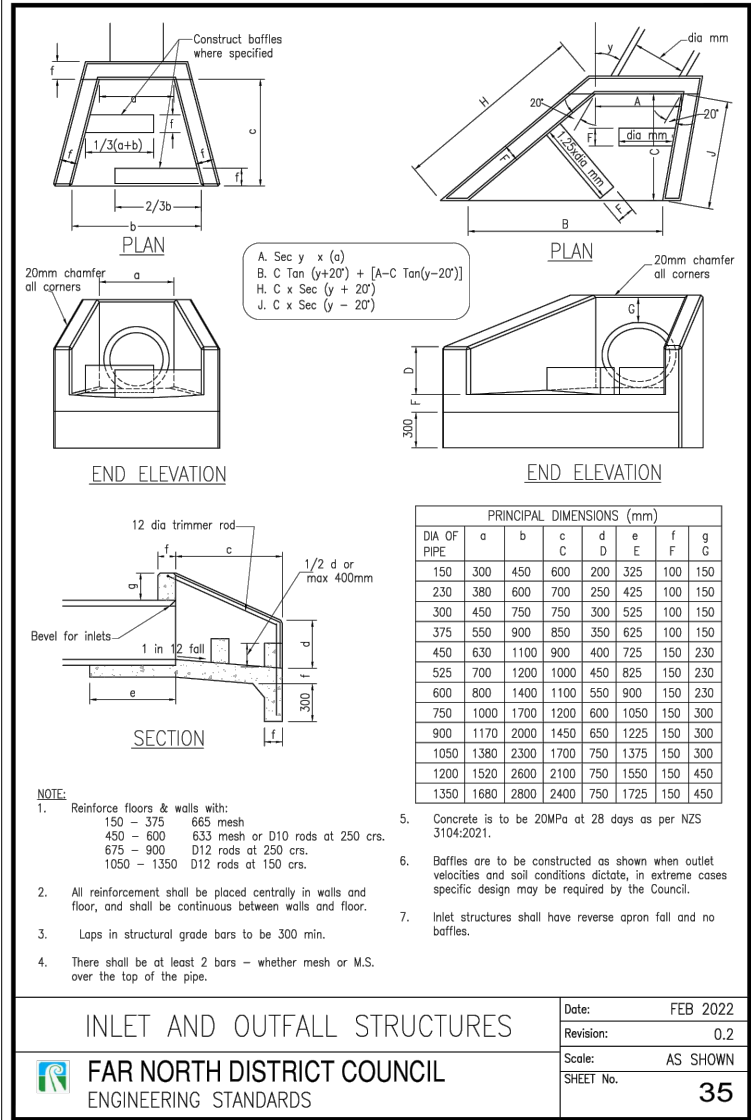
Client  
MOA EQUITY LTD

Sheet Title  
TYPICAL CROSS SECTIONS

Sheet


390

Sheet 35 Inlet and Outlet Structures





## APPENDIX C


### Calculations

Project Ref:	C0651N	PEAK RUNOFF ASSESSMENT				 <b>geologix</b> consulting engineers	
Project Address:	458A HIHI ROAD, HIHI						
Prepared By:	JGM		CATCHMENT A - PROPOSED LOTS 1-2 & PART ROW				
Date:	1 July 2025	REV 1					
ATTENUATION DESIGN PROVIDED IN ACCORDANCE WITH NEW ZEALAND BUILDING CODE E1 FOR THE RATIONALE METHOD ACCOUNTING FOR THE EFFECTS OF CLIMATE CHANGE (20% FACTOR AS PER 2023 FNDC ENGINEERING STANDARDS).							
RUNOFF COEFFICIENTS DETERMINED FROM FNDC ENGINEERING STANDARDS 2023 TABLE 4-3.							
PREDEVELOPMENT SCENARIO				POST DEVELOPMENT SCENARIO			
ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION	ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION
	4870	0.67	PASTURE/ GRASS		4230	0.67	PASTURE/ GRASS
	510	0.85	EXISTING ACCESSWAY		510	0.96	PROPOSED RIGHT OF WAY
					640	0.96	PROPOSED LOT ROOF/PAVING
TOTAL	5380	TYPE D	PR = PROPOSED	TOTAL	5380	TYPE D	
PRE TO POST DEVELOPMENT RUNOFF - 20 % AEP							
20 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			74.4	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
20 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			89.3	mm/hr			
20 % AEP PRE DEVELOPMENT PEAK FLOW			91.67	l/s			
20 % AEP POST DEVELOPMENT PEAK FLOW			97.66	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			5.99	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 50 % AEP							
50 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			57.3	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
50 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			68.8	mm/hr			
50 % AEP PRE DEVELOPMENT PEAK FLOW			70.60	l/s			
50 % AEP POST DEVELOPMENT PEAK FLOW			75.22	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			4.62	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 10 % AEP							
10 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			87.1	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
10 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			104.5	mm/hr			
10 % AEP PRE DEVELOPMENT PEAK FLOW			107.32	l/s			
10 % AEP POST DEVELOPMENT PEAK FLOW			114.34	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			7.02	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 1 % AEP							
1 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			132.0	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
1 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			158.4	mm/hr			
1 % AEP PRE DEVELOPMENT PEAK FLOW			154.32	l/s			
1 % AEP POST DEVELOPMENT PEAK FLOW			173.28	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			18.96	l/s			



Project Ref:	C0651N		PEAK RUNOFF ASSESSMENT		 <b>geologix</b> consulting engineers		
Project Address:	458A HIHI ROAD, HIHI						
Prepared By:	GM		CATCHMENT B - PART ROW				
Date:	1 July 2025	REV 1					
ATTENUATION DESIGN PROVIDED IN ACCORDANCE WITH NEW ZEALAND BUILDING CODE E1 FOR THE RATIONALE METHOD ACCOUNTING FOR THE EFFECTS OF CLIMATE CHANGE (20% FACTOR AS PER 2023 FNDC ENGINEERING STANDARDS).							
RUNOFF COEFFICIENTS DETERMINED FROM FNDC ENGINEERING STANDARDS 2023 TABLE 4-3.							
PREDEVELOPMENT SCENARIO				POST DEVELOPMENT SCENARIO			
ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION	ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION
	1195	0.67	PASTURE/ GRASS		1195	0.67	PASTURE/ GRASS
	510	0.85	EXISTING ACCESSWAY		510	0.96	PROPOSED RIGHT OF WAY
							PROPOSED LOT ROOF/PAVING
TOTAL	1705	TYPE D	PR = PROPOSED	TOTAL	1705	TYPE D	
PRE TO POST DEVELOPMENT RUNOFF - 20 % AEP							
20 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			74.4	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
20 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			89.3	mm/hr			
20 % AEP PRE DEVELOPMENT PEAK FLOW			30.61	l/s			
20 % AEP POST DEVELOPMENT PEAK FLOW			32.00	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.39	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 50 % AEP							
50 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			57.3	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
50 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			68.8	mm/hr			
50 % AEP PRE DEVELOPMENT PEAK FLOW			23.57	l/s			
50 % AEP POST DEVELOPMENT PEAK FLOW			24.64	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.07	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 10 % AEP							
10 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			87.1	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
10 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			104.5	mm/hr			
10 % AEP PRE DEVELOPMENT PEAK FLOW			35.83	l/s			
10 % AEP POST DEVELOPMENT PEAK FLOW			37.46	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.63	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 1 % AEP							
1 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			132.0	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
1 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			158.4	mm/hr			
1 % AEP PRE DEVELOPMENT PEAK FLOW			54.30	l/s			
1 % AEP POST DEVELOPMENT PEAK FLOW			56.77	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			2.47	l/s			

Project Ref:	C0651N		PEAK RUNOFF ASSESSMENT		<div></div> <div>geologix consulting engineers</div>		
Project Address:	458A HIHI ROAD, HIHI						
Prepared By:	JGM		CATCHMENT C - LOTS 3-6				
Date:	1 July 2025	REV 1					
ATTENUATION DESIGN PROVIDED IN ACCORDANCE WITH NEW ZEALAND BUILDING CODE E1 FOR THE RATIONALE METHOD ACCOUNTING FOR THE EFFECTS OF CLIMATE CHANGE (20% FACTOR AS PER 2023 FNDC ENGINEERING STANDARDS).							
RUNOFF COEFFICIENTS DETERMINED FROM FNDC ENGINEERING STANDARDS 2023 TABLE 4-3.							
PREDEVELOPMENT SCENARIO				POST DEVELOPMENT SCENARIO			
ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION	ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION
	30142	0.67	PASTURE/ GRASS		28662	0.67	PASTURE/ GRASS
	0	0.96	EXISTING ACCESSWAY		0	0.96	PROPOSED RIGHT OF WAY
					1480	0.96	PROPOSED LOT ROOF/PAVING
TOTAL	30142	TYPE D	PR = PROPOSED	TOTAL	30142	TYPE D	
PRE TO POST DEVELOPMENT RUNOFF - 20 % AEP							
20 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			74.4	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
20 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			89.3	mm/hr			
20 % AEP PRE DEVELOPMENT PEAK FLOW			500.84	l/s			
20 % AEP POST DEVELOPMENT PEAK FLOW			511.48	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			10.64	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 50 % AEP							
50 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			57.3	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			25.62	%			
50 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			72.0	mm/hr			
50 % AEP PRE DEVELOPMENT PEAK FLOW			403.79	l/s			
50 % AEP POST DEVELOPMENT PEAK FLOW			412.37	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			8.58	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 10 % AEP							
10 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			87.1	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			27.51	%			
10 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			111.1	mm/hr			
10 % AEP PRE DEVELOPMENT PEAK FLOW			623.03	l/s			
10 % AEP POST DEVELOPMENT PEAK FLOW			636.27	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			13.24	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 1 % AEP							
1 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			132.0	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			28.56	%			
1 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			169.7	mm/hr			
1 % AEP PRE DEVELOPMENT PEAK FLOW			951.97	l/s			
1 % AEP POST DEVELOPMENT PEAK FLOW			972.20	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			20.23	l/s			

Project Ref:	C0651N		PEAK RUNOFF ASSESSMENT	 <b>geologix</b> consulting engineers
Project Address:	458A HIHI ROAD, HIHI			
Prepared By:	GM		CATCHMENT D - BALANCE OF LOT 1	
Date:	1 July 2025	REV 1		

ATTENUATION DESIGN PROVIDED IN ACCORDANCE WITH NEW ZEALAND BUILDING CODE E1 FOR THE RATIONALE METHOD ACCOUNTING FOR THE EFFECTS OF CLIMATE CHANGE (20% FACTOR AS PER 2023 FNDC ENGINEERING STANDARDS).

RUNOFF COEFFICIENTS DETERMINED FROM FNDC ENGINEERING STANDARDS 2023 TABLE 4-3.


PREDEVELOPMENT SCENARIO				POST DEVELOPMENT SCENARIO			
ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION	ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION
	5179	0.67	PASTURE/ GRASS		4859	0.67	PASTURE/ GRASS
	545	0.85	EXISTING ACCESSWAY		545	0.96	PROPOSED RIGHT OF WAY
					320	0.96	PROPOSED LOT ROOF/PAVING
TOTAL	5724	TYPE D	PR = PROPOSED	TOTAL	5724	TYPE D	

PRE TO POST DEVELOPMENT RUNOFF - 20 % AEP			
20 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr	74.4	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS	20	%	
20 % AEP RAINFALL INTENSITY, 10 MIN WITH CC	89.3	mm/hr	
20 % AEP PRE DEVELOPMENT PEAK FLOW	97.54	l/s	
20 % AEP POST DEVELOPMENT PEAK FLOW	101.33	l/s	
PRE TO POST DEVELOPMENT DIFFERENCE	3.79	l/s	

PRE TO POST DEVELOPMENT RUNOFF - 50 % AEP			
50 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr	57.3	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS	25.62	%	
50 % AEP RAINFALL INTENSITY, 10 MIN WITH CC	72.0	mm/hr	
50 % AEP PRE DEVELOPMENT PEAK FLOW	78.64	l/s	
50 % AEP POST DEVELOPMENT PEAK FLOW	81.70	l/s	
PRE TO POST DEVELOPMENT DIFFERENCE	3.05	l/s	

PRE TO POST DEVELOPMENT RUNOFF - 10 % AEP			
10 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr	87.1	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS	27.51	%	
10 % AEP RAINFALL INTENSITY, 10 MIN WITH CC	111.1	mm/hr	
10 % AEP PRE DEVELOPMENT PEAK FLOW	121.34	l/s	
10 % AEP POST DEVELOPMENT PEAK FLOW	126.05	l/s	
PRE TO POST DEVELOPMENT DIFFERENCE	4.71	l/s	

PRE TO POST DEVELOPMENT RUNOFF - 1 % AEP			
1 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr	132.0	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS	28.56	%	
1 % AEP RAINFALL INTENSITY, 10 MIN WITH CC	169.7	mm/hr	
1 % AEP PRE DEVELOPMENT PEAK FLOW	185.40	l/s	
1 % AEP POST DEVELOPMENT PEAK FLOW	192.61	l/s	
PRE TO POST DEVELOPMENT DIFFERENCE	7.20	l/s	

Project Ref:	C0651N		PEAK RUNOFF ASSESSMENT				 <b>geologix</b> consulting engineers
Project Address:	458A HIHI ROAD, HIHI						
Prepared By:	JGM		CATCHMENT E - PART ROW				
Date:	1 July 2025	REV 1					
ATTENUATION DESIGN PROVIDED IN ACCORDANCE WITH NEW ZEALAND BUILDING CODE E1 FOR THE RATIONALE METHOD ACCOUNTING FOR THE EFFECTS OF CLIMATE CHANGE (20% FACTOR AS PER 2023 FNDC ENGINEERING STANDARDS).							
RUNOFF COEFFICIENTS DETERMINED FROM FNDC ENGINEERING STANDARDS 2023 TABLE 4-3.							
PREDEVELOPMENT SCENARIO				POST DEVELOPMENT SCENARIO			
ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION	ITEM	AREA, A, m2	COEFFICIENT, C	DESCRIPTION
	397	0.67	PASTURE/ GRASS		397	0.67	PASTURE/ GRASS
	545	0.85	EXISTING ACCESSWAY		545	0.96	PROPOSED RIGHT OF WAY
							PROPOSED LOT ROOF/PAVING
TOTAL	942	TYPE D	PR = PROPOSED	TOTAL	942	TYPE D	
PRE TO POST DEVELOPMENT RUNOFF - 20 % AEP							
20 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			74.4	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			20	%			
20 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			89.3	mm/hr			
20 % AEP PRE DEVELOPMENT PEAK FLOW			18.09	l/s			
20 % AEP POST DEVELOPMENT PEAK FLOW			19.57	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.49	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 50 % AEP							
50 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			57.3	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			25.62	%			
50 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			72.0	mm/hr			
50 % AEP PRE DEVELOPMENT PEAK FLOW			14.58	l/s			
50 % AEP POST DEVELOPMENT PEAK FLOW			15.78	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.20	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 10 % AEP							
10 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			87.1	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			27.51	%			
10 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			111.1	mm/hr			
10 % AEP PRE DEVELOPMENT PEAK FLOW			22.50	l/s			
10 % AEP POST DEVELOPMENT PEAK FLOW			24.35	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			1.85	l/s			
PRE TO POST DEVELOPMENT RUNOFF - 1 % AEP							
1 % AEP RAINFALL INTENSITY, 10 MIN, I, mm/hr			132.0	mm/hr	* CLIMATE CHANGE FACTOR OF 20% APPLIED IN ACCORDANCE WITH FNDC ENGINEERING STANDARDS 4.3.9.1. NIWA HISTORIC RAINFALL INTENSITY DATA, 10MIN, IS MULTIPLIED BY CLIMATE CHANGE FACTOR.		
CLIMATE CHANGE FACTOR, 20% AS PER FNDC STANDARDS			28.56	%			
1 % AEP RAINFALL INTENSITY, 10 MIN WITH CC			169.7	mm/hr			
1 % AEP PRE DEVELOPMENT PEAK FLOW			34.38	l/s			
1 % AEP POST DEVELOPMENT PEAK FLOW			37.20	l/s			
PRE TO POST DEVELOPMENT DIFFERENCE			2.83	l/s			

HIRDS V4 Intensity-Duration-Frequency Results

SiteName: CHHI

Coordinate system: WGS84

Longitude: 173.5479

Latitude: -34.971

DOF ModelParameters: c d e f g h i  
Values: 0.001702 0.504677 -0.03943 0 0.25324 -0.01065 3.196961

Example: Duration (hAR) (yrs) x Rainfall Rate (mm/hr)  
24 100 3.178054 4.600149 8.92641

Rainfall Intensities (mm/hr) :: Historical Data

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	52.3	40.2	33.8	24.4	17	8.87	5.5	3.4	1.99	1.43	1.12	0.924
2	0.5	57.3	44	37.1	26.8	18.7	9.73	6.14	3.74	2.19	1.57	1.23	1.03
5	0.2	74.4	57.2	48.2	34.9	24.3	12.7	8.04	4.9	2.87	2.06	1.62	1.34
10	0.1	87.1	67	56.5	41	28.6	15	9.47	5.77	3.38	2.43	1.91	1.58
20	0.05	100	77.2	65.1	47.2	33	17.3	11	6.68	3.92	2.82	2.22	1.83
30	0.033	108	83.2	70.3	51.9	35.7	18.7	11.8	7.23	4.25	3.06	2.4	1.98
40	0.025	114	87.7	74	53.7	37.6	19.7	12.5	7.63	4.48	3.23	2.54	2.09
50	0.02	118	91.1	76.9	55.9	39.1	20.5	13	7.94	4.67	3.36	2.64	2.18
60	0.017	122	94	79.3	57.6	40.3	21.2	13.4	8.2	4.82	3.47	2.73	2.25
80	0.013	128	98.5	83.1	60.4	42.3	22.2	14.1	8.61	5.06	3.65	2.87	2.37
100	0.01	132	102	86.1	62.6	43.8	23	14.6	8.93	5.25	3.78	2.97	2.46
250	0.004	151	116	98.3	71.5	50.1	26.4	16.8	10.3	6.04	4.35	3.42	2.83

Intensity standard error (mm/hr) :: Historical Data

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	6.7	4.4	3.2	2.4	1.7	1	0.71	0.56	0.36	0.27	0.21	0.18
2	0.5	7.3	4.8	3.5	2.6	1.8	1.1	0.78	0.63	0.4	0.31	0.23	0.2
5	0.2	10	6.9	5.2	3.7	2.6	1.6	1.1	0.84	0.53	0.41	0.31	0.27
10	0.1	13	9.1	6.9	4.9	3.4	2	1.3	1	0.64	0.49	0.37	0.32
20	0.05	16	12	9.2	6.5	4.5	2.6	1.7	1.2	0.75	0.58	0.44	0.37
30	0.033	19	14	11	7.6	5.3	3	2	1.3	0.83	0.64	0.48	0.41
40	0.025	21	16	12	8.5	6	3.4	2	1.4	0.88	0.68	0.52	0.44
50	0.02	23	17	13	9.3	6.6	3.7	2.4	1.5	0.93	0.72	0.54	0.46
60	0.017	24	18	14	10	7.1	4	2.6	1.5	0.97	0.75	0.57	0.48
80	0.013	27	20	16	11	7.9	4.4	2.9	1.7	1	0.8	0.61	0.51
100	0.01	29	22	17	12	8.7	4.8	3.1	1.7	1.1	0.84	0.64	0.54
250	0.004	39	31	24	17	12	6.9	4.5	2.2	1.3	1	0.79	0.66

Rainfall Intensities (mm/hr) :: RCP2.6 for the period 2031-2050

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	56	43	36.2	26.2	18.1	9.36	5.86	3.54	2.06	1.47	1.15	0.949
2	0.5	61.4	47.2	39.7	28.7	20	10.3	6.45	3.89	2.27	1.62	1.27	1.05
5	0.2	80	61.5	51.8	37.5	26.1	13.5	8.48	5.12	2.98	2.14	1.67	1.38
10	0.1	93.8	72.2	60.9	44.1	30.7	15.9	10	6.05	3.52	2.53	1.98	1.63
20	0.05	108	83.2	70.2	50.9	35.5	18.4	11.6	7	4.09	2.93	2.3	1.89
30	0.033	117	89.9	75.8	55	38.4	19.9	12.5	7.58	4.43	3.18	2.49	2.05
40	0.025	123	94.6	79.8	58	40.4	21	13.2	8	4.67	3.36	2.63	2.17
50	0.02	128	98.4	83	60.3	42.1	21.9	13.8	8.33	4.87	3.49	2.74	2.26
60	0.017	131	101	85.6	62.2	43.4	22.6	14.2	8.61	5.03	3.61	2.83	2.33
80	0.013	138	106	89.8	65.3	45.5	23.7	14.9	9.04	5.29	3.79	2.97	2.45
100	0.01	143	110	93	67.6	47.2	24.6	15.5	9.38	5.48	3.94	3.09	2.55
250	0.004	163	126	106	77.3	54	28.2	17.8	10.8	6.31	4.53	3.56	2.93

Rainfall Intensities (mm/hr) :: RCP2.6 for the period 2081-2100

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	56	43	36.2	26.2	18.1	9.36	5.86	3.54	2.06	1.47	1.15	0.949
2	0.5	61.4	47.2	39.7	28.7	20	10.3	6.45	3.89	2.27	1.62	1.27	1.05
5	0.2	80	61.5	51.8	37.5	26.1	13.5	8.48	5.12	2.98	2.14	1.67	1.38
10	0.1	93.8	72.2	60.9	44.1	30.7	15.9	10	6.05	3.52	2.53	1.98	1.63
20	0.05	108	83.2	70.2	50.9	35.5	18.4	11.6	7	4.09	2.93	2.3	1.89
30	0.033	117	89.9	75.8	55	38.4	19.9	12.5	7.58	4.43	3.18	2.49	2.05
40	0.025	123	94.6	79.8	58	40.4	21	13.2	8	4.67	3.36	2.63	2.17
50	0.02	128	98.4	83	60.3	42.1	21.9	13.8	8.33	4.87	3.49	2.74	2.26
60	0.017	131	101	85.6	62.2	43.4	22.6	14.2	8.61	5.03	3.61	2.83	2.33
80	0.013	138	106	89.8	65.3	45.5	23.7	14.9	9.04	5.29	3.79	2.97	2.45
100	0.01	143	110	93	67.6	47.2	24.6	15.5	9.38	5.48	3.94	3.09	2.55
250	0.004	163	126	106	77.3	54	28.2	17.8	10.8	6.31	4.53	3.56	2.93

Rainfall Intensities (mm/hr) :: RCP2.6 for the period 2031-2050

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	56.9	43.7	36.8	26.6	18.4	9.48	5.93	3.58	2.08	1.48	1.16	0.955
2	0.5	62.5	48	40.4	29.2	20.3	10.4	6.53	3.93	2.29	1.63	1.28	1.05
5	0.2	81.4	62.6	52.8	38.2	26.6	13.7	8.59	5.18	3.01	2.16	1.69	1.39
10	0.1	95.1	73.5	62	44.9	31.3	16.2	10.1	6.12	3.56	2.55	1.95	1.64
20	0.05	110	84.8	71.5	51.9	36.1	18.7	11.7	7.08	4.13	2.96	2.32	1.91
30	0.033	119	91.5	77.2	56.1	39.1	20.2	12.7	7.67	4.48	3.21	2.51	2.07
40	0.025	125	96.4	81.3	59.1	41.2	21.4	13.4	8.1	4.72	3.39	2.65	2.19
50	0.02	130	100	84.6	61.4	42.8	22.2	14	8.43	4.92	3.53	2.77	2.28
60	0.017	134	103	87.2	63.4	44.2	22.9	14.4	8.71	5.08	3.65	2.86	2.35
80	0.013	140	108	91.5	66.5	46.4	24.1	15.1	9.15	5.34	3.83	3	2.47
100	0.01	145	112	94.8	68.9	48	25	15.7	9.49	5.54	3.98	3.12	2.57
250	0.004	166	128	108	78.7	55	28.6	18	10.9	6.37	4.58	3.59	2.96

Rainfall Intensities (mm/hr) :: RCP4.5 for the period 2081-2100

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	59.9	45.9	38.7	28	19.3	9.88	6.14	3.69	2.13	1.52	1.18	0.975
2	0.5	65.7	50.5	42.5	30.8	21.3	10.9	6.78	4.06	2.35	1.68	1.31	1.08
5	0.2	85.9	66.1	55.7	40.3	28	14.3	8.94	5.36	3.1	2.22	1.73	1.42
10	0.1	101	77.7	65.5	47.5	33	16.9	10.6	6.34	3.67	2.63	2.05	1.69
20	0.05	115	89.6	75.6	54.8	38.1	19.6	12.2	7.34	4.26	3.05	2.38	1.96
30	0.033	126	96.8	81.7	59.3	41.2	21.2	13.3	7.95	4.62	3.3	2.58	2.12
40	0.025	132	102	86	62.4	43.4	22.4	14	8.4	4.88	3.49	2.73	2.25
50	0.02	137	106	89.5	65	45.2	23.3	14.6	8.74	5.08	3.64	2.85	2.34
60	0.017	142	109	92.3	67	46.6	24.1	15	9.04	5.25	3.76	2.94	2.42
80	0.013	149	115	96.8	70.4	49	25.3	15.8	9.49	5.52	3.95	3.09	2.54
100	0.01	154	119	100	72.9	50.7	26.2	16.4	9.86	5.73	4.1	3.21	2.64
250	0.004	175	135	114	83.3	58.1	30.1	18.8	11.3	6.59	4.72	3.69	3.04

Rainfall Intensities (mm/hr) :: RCP6.0 for the period 2031-2050

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	56.6	43.4	36.5	26.4	18.3	9.43	5.9	3.56	2.07	1.48	1.16	0.953
2	0.5	62	47.7	40.1	29	20.1	10.4	6.5	3.92	2.28	1.63	1.28	1.05
5	0.2	80.9	62.2	52.4	38	26.4	13.6	8.55	5.16	3	2.15	1.68	1.39
10	0.1	94.9	73	61.6	44.6	31	16.1	10.1	6.09	3.55	2.54	1.99	1.64
20	0.05	109	84.2	71	51.5	35.9	18.6	11.7	7.05	4.11	2.95	2.31	1.9
30	0.033	118	90.9	76.7	55.7	38.8	20.1	12.6	7.64	4.46	3.2	2.5	2.06
40	0.025	124	95.7	80.7	58.6	40.9	21.2	13.3	8.06	4.7	3.38	2.64	2.18
50	0.02	129	99.5	84	61	42.5	22.1	13.9	8.39	4.9	3.52	2.76	2.27
60	0.017	133	103	86.6	62.9	43.9	22.8	14.3	8.67	5.06	3.63	2.85	2.34
80	0.013	139	108	90.8	66	46	23.9	15	9.1	5.32	3.82	2.99	2.47
100	0.01	144	111	94.1	68.4	47.7	24.8	15.6	9.45	5.52	3.96	3.11	2.56
250	0.004	164	127	107	78.2	54.6	28.5	17.9	10.9	6.35	4.56	3.58	2.95

Rainfall Intensities (mm/hr) :: RCP6.0 for the period 2081-2100

ARI	AEP	10m	20m	30m	1h	2h	6h	12h	24h	48h	72h	96h	120h
1.58	0.633	62.5	48.8	40.4	29.2	20.1	10.2	6.33	3.7	2.18	1.51	1.21	0.992
2	0.5	68.7	52.4	44.4	32.1	22.2	11.3	7	4.17	2.4	1.71	1.31	1.01
5	0.2	89.9	69.1	58.3	42.2	29.2	14.9	9.25	5.52	3.18	2.27	1.77	1.45
10	0.1	106	86.4	68.6	49.7	34.5	17.6	10.9	6.53	3.77	2.69	2.1	1.72
30	0.05	122	93.9	79.2	57.5	39.9	20.4	12.7	7.57	4.38	3.13	2.44	2.07
60	0.025	131	101	85.6	62.1	43.2	23.7	13.7	8.21	4.75	3.48	2.7	2.23
90	0.025	139	107	90.2	65.5	45.5	23.3	14.5	8.67	5.01	3.58	2.8	2.33
80	0.02	144	111	93.8	68.2	47.3	24.3	15.1	9.02	5.23	3.73	2.92	2.39
90	0.017	149	115	96.8	70.3	48.8	25.1	15.6	9.33	5.4	3.86	3.01	2.47
100	0.017	156	119	100	72.5	51.3	26.3	16.4	9.68	5.68	4.04	3.16	2.57
100	0.01	161	125	105	75	53.2	27.3	17	10.2	5.89	4.21	3.29	2.7
250	0.004	184	142	120	87.4	60.8	31.3	19.5	11.7	6.78	4.84	3.79	3.11

## Colebrook-White Calculation

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	<b>Project:</b>	458A HIHI ROAD, HIHI		<b>By:</b>	GM
	<b>Calculation:</b>	Stormwater Pipe Analysis		<b>Approved:</b>	SH
	Colebrook-White Pipe Analysis				
	<b>Sheet Number:</b>	1	<b>Date:</b>	18-Jul	

<b>General Info</b>		
Kinematic Viscosity ( $\nu$ )	1.139	mm <sup>2</sup> /s
Gravity ( $g$ )	9.80665	m/s <sup>2</sup>

<b>Pipe Info</b>		
Roughness Coefficient ( $k_s$ )	0.6	mm
Hydraulic Gradient ( $S$ )	1	%
Internal Diameter ( $D$ )	375	mm

= 0.01 m/m  
or 1 in 100

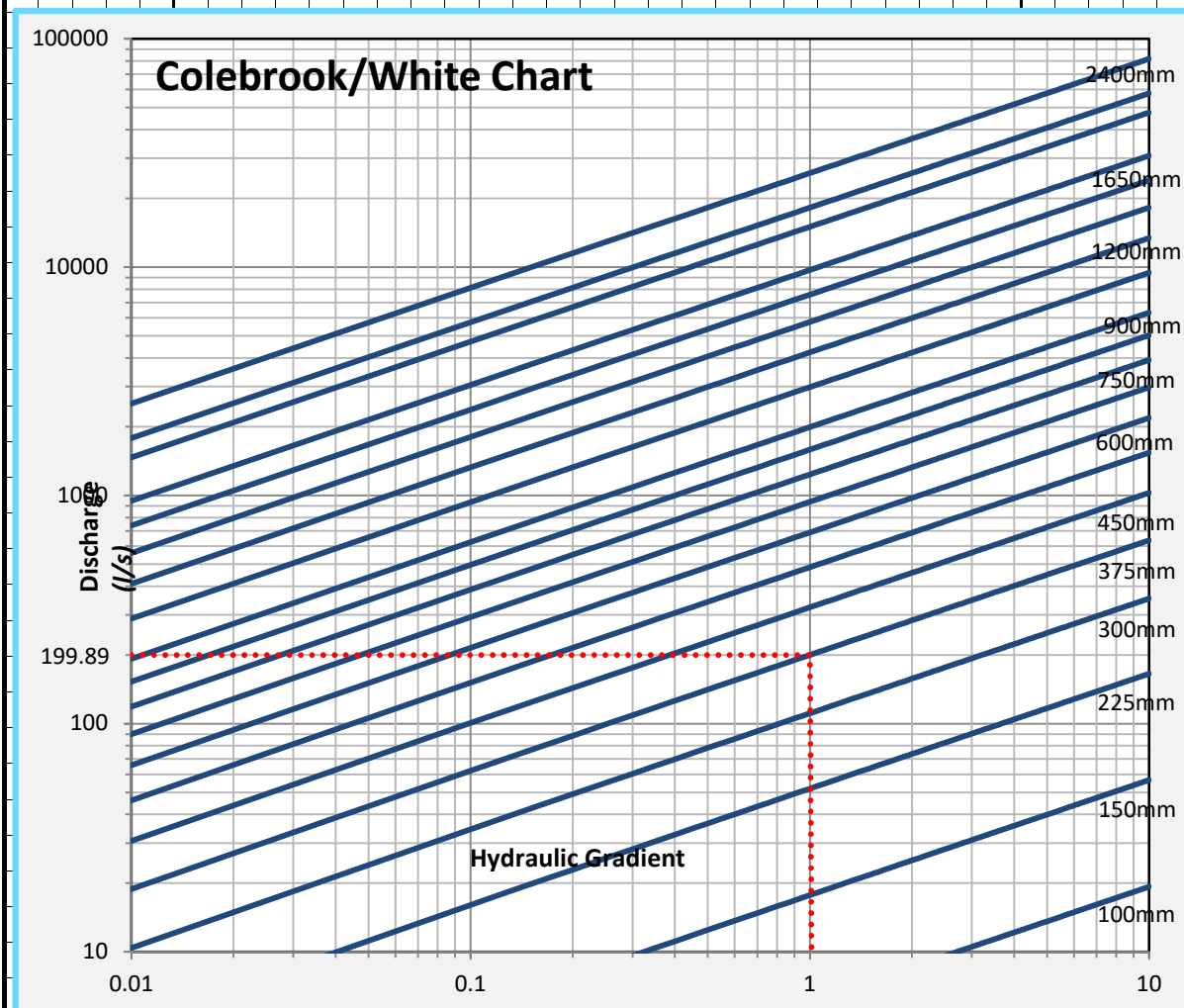
  

$$Q = -2\sqrt{2gDS} \times \log \left[ \frac{k_s}{3.7D} + \frac{2.51\nu}{D\sqrt{2gDS}} \right] A$$

<b>Results When Flowing Full</b>		
Velocity ( $v$ )	1.81	m/s
Flow ( $Q$ )	199.89	l/s

= 0.20 m<sup>3</sup>/s





# Colebrook-White Calculation



<b>Project:</b>	458A HIHI ROAD, HIHI	<b>By:</b>	
<b>Calculation:</b>	Stormwater Pipe Analysis	<b>GM</b>	
<b>Sheet Number:</b>	2	<b>Date:</b>	18-Jul
		<b>Approved:</b>	SH

## Results When Flowing Partially Full

Max Velocity at 81.4% Height

Vmax **2.06** m/s

Max Flow at 93.8% Height

Qmax **215.0** l/s

= 0.22 m<sup>3</sup>/s

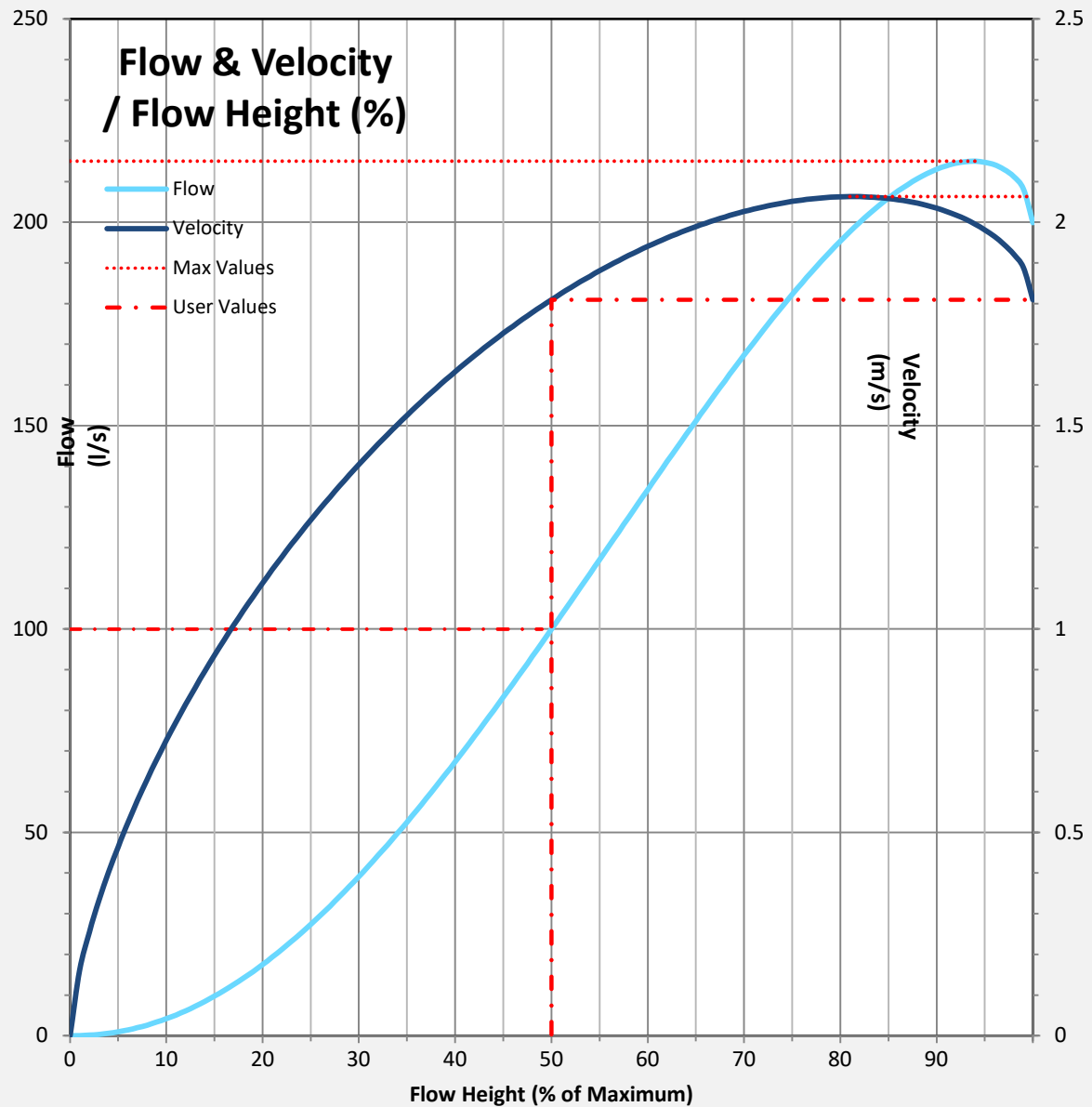
User Defined Height **50** %

= 0.188 m

Velocity (v) **1.81** m/s

Flow (Q) **99.9** l/s

= 0.10 m<sup>3</sup>/s



## Colebrook-White Calculation

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<b>Project:</b>	458A HIHI ROAD, HIHI	<b>By:</b>	
<b>Calculation:</b>	SW Pipe RoW	<b>GM</b>	
Colebrook-White Pipe Design			
<b>Sheet Number:</b>	1	<b>Date:</b>	1-Jul
		<b>Approved:</b>	SH

### General Info

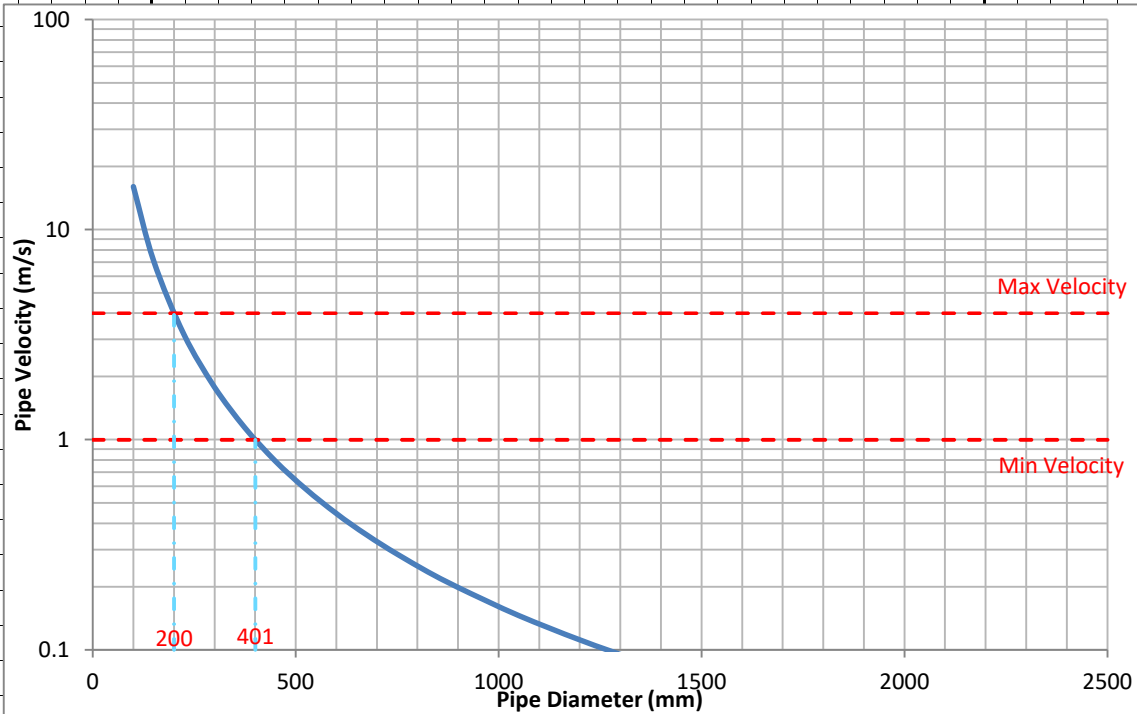
Kinematic Viscosity ( $\nu$ )	1.139	mm <sup>2</sup> /s
Gravity ( $g$ )	9.80665	m/s <sup>2</sup>

### Pipe Info

Roughness Coefficient ( $k_s$ )	0.6	mm
---------------------------------	-----	----

Required Flow	126.05	l/s
Min Velocity	1	m/s
Max Velocity	4	m/s

= 0.13 m<sup>3</sup>/s



### Suggested Pipe Sizes and Gradients

	Pipe Diameter	Full Flow Velocity (m/s)	Required Gradient (%)	Required Gradient (1 in X)
1	225	3.17	5.84	17.11
2	300	1.78	1.29	77.30
3	375	1.14	0.40	247.64
4				
5				
6				
7				
8				
9				
10				

# Colebrook-White Calculation



<b>Project:</b>	458A HIHI ROAD, HIHI	<b>By:</b>	
<b>Calculation:</b>	SW Pipe RoW	<b>GM</b>	
		<b>Approved:</b>	
<b>Sheet Number:</b>	2	<b>Date:</b>	1-Jul
		<b>SH</b>	

## Chosen Pipe Analysis

Pipe Diameter	375	mm
Pipe Gradient	2.75	%

= 0.03 m/m  
or 1 in 36.4

Max Pipe Capacity at Full Flow:	332.5 l/s	Acceptable
Max Flow at 93.8% Height:	357.7 l/s	Acceptable
Velocity at Full Flow:	3.01 m/s	Acceptable
Actual Velocity at 126.05 l/s:	2.78 m/s	Acceptable
Max Velocity at 81.4% Height	3.43 m/s	Acceptable

= 0.33 m³/s  
= 0.36 m³/s

